

	DIMENSIONS FOR I-BEAM PADS								
PAD	А	В	С	*MAXIMUM REACTION	MAXIMUM MOVEMENT (One Direction)				
1F	14"	10"	2~0.12" × 13.630" × 9.630"	121k	0.5"				
2F	16"	10"	2~0.12" × 15.630" × 9.630"	146k	0.5"				
3F	20"	10"	2~0.12" × 19.630" × 9.630"	197k	0.5"				
4F	24"	10"	2~0.12" × 23.630" × 9.630"	251k	0.5"				
5F	24"	11"	2~0.12" × 23.630" × 10.630"	295k	0.5"				

\* Use actual reactions to determine anchorage requirements for pads.

	DIMENSIONS FOR I-BEAM PADS								
PAD	А	В	С	*MAXIMUM REACTION	MAXIMUM MOVEMENT (One Direction)				
1E	14"	10"	6~0.12" × 13.630" × 9.630"	121k	1.22"				
2E	16"	10"	6~0.12" × 15.630" × 9.630"	146k	1.22"				
3E	20"	10"	6~0.12" × 19.630" × 9.630"	197k	1.22"				
4E	24"	10"	6~0.12" × 23.630" × 9.630"	251k	1.22"				
5E	24"	11"	7~0.12" x 23.630" x 10.630"	295k	1.44"				

\* These reactions are based on service loads, use actual reactions to determine anchorage requirements for pads.

#### GENERAL NOTES

SPECIFICATIONS: Fabricate the Elastomeric Bearing Pads to the design and dimensions as shown on these drawings and to AASHTO LRFD Bridge Construction Specifications, Section 18.

Ensure bearings are low temperature Grade 3 with durometer hardness of 50 and subjected to the load testing requirements corresponding to Design Method A.

Include the price of bearing pads in the bid for the beams.

KENTUCKY DEPARTMENT OF HIGHWAYS **ELASTOMERIC BEARING** 

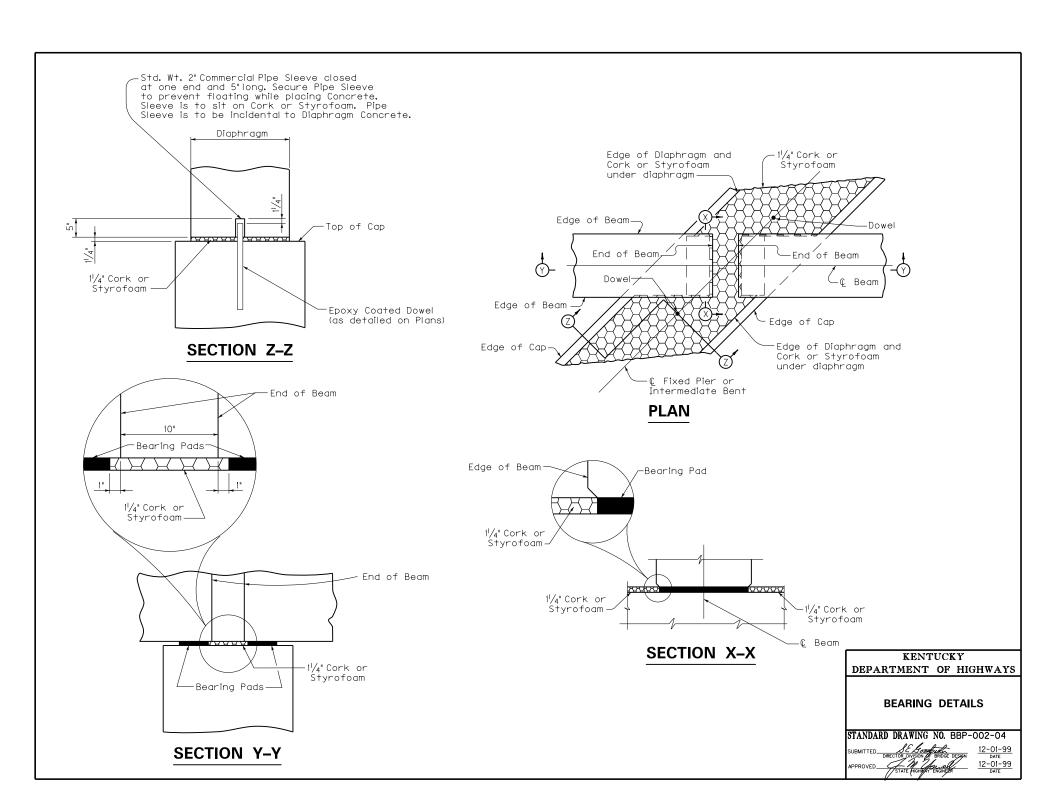
PADS FOR

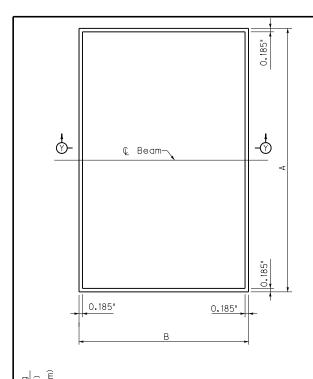
PRESTRESSED BEAMS

12-01-11

STANDARD DRAWING NO. BBP-001-12 12-01-11

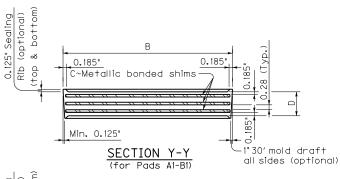
Man July DIRECTOR DIVISION OF STRUCTURAL DESIGN

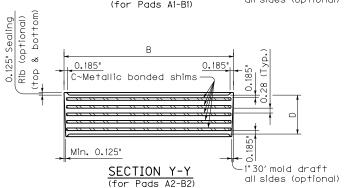




	DIMENSIONS FOR BOX-BEAM PADS							
PAD	А	В	С	D	*MAXIMUM REACTION	MAXIMUM MOVEMENT (One Direction)		
Α1	1′-10"	7"	3~0.12" × 21.630" × 6.630"	1.290"	173k	0.500"		
A2	1′-10"	7"	5~0.12" × 21.630" × 6.630"	2.090"	173k	0.750"		
B1	11"	7"	3~0.12" × 10.630" × 6.630"	1.290"	69k	0.500"		
B2	11"	7"	5~0.12" × 10.630" × 6.630"	2.090"	69k	0.750"		

\* These reactions are based on service loads, use actual reactions to determine anchorage requirements for pads.





#### GENERAL NOTES

SPECIFICATIONS: Fabricate the Elastomeric Bearing Pads to the design and dimensions as shown on these drawings and to AASHTO LRFD Bridge Construction Specifications, Section 18.

Ensure bearings are low temperature Grade 3 with durometer hardness of 50 and subjected to the load testing requirements corresponding to Design Method A.

Include the price of bearing pads in the bid for the beams.

#### KENTUCKY DEPARTMENT OF HIGHWAYS

**ELASTOMERIC BEARING** PADS FOR **BOX BEAMS** 

STANDARD DRAWING NO. BBP-003-02

Man Jute

DIRECTOR DISSIPATION OF STRUCTURAL DESIGN 12-01-11 DATE STATE HIGHWAY ENGINEER

12-01-11

## PRECAST PRESTRESSED BOX BEAMS

#### General Notes

SPECIFICATIONS: All references to the standard Specifications are to the current edition of the Kentucky Department of Highways Standard Specifications for Road and Bridge Construction, with current supplemental specifications. All references to the AASHTO Specifications are to the current edition of the AASHTO LRFD Bridge Design Specifications, with interims.

DESIGN LOADS: Beam sections are designed for 1.25\*HL93 (KYHL93) Live Load.

DESIGN LOAD DISTRIBUTION: Contrary to AASHTO LRFD Bridge Design Specifications, the design moment and shear distribution for all beams is 0.5 lanes.

FUTURE WEARING SURFACE: These beams are designed for a 15 PSF future wearing surface load.

SUBSTRUCTURE DESIGN LOADS: Unfactored design reaction forces per beam end.

- DC (kips): Beam, Slab (if applicable), and Type II railing dead loads.
- DW (kips): Future wearing surface.
- LL (kips): Beam Live Load reaction per lane x Design load distribution.
- LL+I (kips): LL with Dynamic load allowance.

#### DESIGN DEFLECTIONS:

- Ad (in.): Sum of the downwards deflections caused by the design 5' deck, railing, and future wearing surface. (Positive Downwards)
- $\Delta c$  (in.): Upwards midspan camber of the beam caused by prestressing minus the downward deflection of the beam due to self weight. (Positive Upwards)

#### MATERIAL DESIGN SPECIFICATIONS:

for	Steel Reinforcement	FY	=	60000	PS
for	Prestressed Girder Concrete	F′C	=	7000	PS
		F'CI	=	5500	PS
for	Class "AA" Concrete	F′C	=	4000	PS
for	Prestressing Steel	F′S	=	270000	PS

DESIGN LENGTH: Beam lengths shown in the Standards represent total beam length. Use the next greater designed section for non-Standard lengths.

CONSTRUCTION METHOD: Transferring bond stress to the concrete will not be allowed, nor releasing of end anchors until the concrete has attained a minimum compressive strength of 5500 PSI as shown by standard cylinders made and cured identically with the girders; attain 7000 PSI at or prior to 28 days. Apply an initial prestress force of 33817 lbs. per low relaxation strand. Beams with honeycomb of such extent as to affect the strength of resistance to deterioration will not be accepted. The allowance of .0005L (length) is made for shortening of beams due to shrinkage and elastic change. Furnish shop plans showing a detensioning plan by numbering, in sequence, the strand pattern.

PRESTRESSING STRANDS: Ensure prestressing strands to be  $\frac{1}{2}$  oversize (0.167 sq. in.) uncoated seven-wire stress relieved, low-relaxation strands conforming to AASHTO M 203, Grade 270. If an alternate strand arrangement or strand type is preferred by the Contractor, the designer that developed the original plans will provide the design and also revise the original plans to reflect the changes. These design and plan modifications will be done at the Contractor's expense.

CORROSION INHIBITOR: Provide a corrosion inhibitor for B-type (non-composite) beams from the list of approved materials.

BEVELED EDGES: Bevel all exposed edges 3/4".

REINFORCEMENT: Dimensions shown from the face of concrete to reinforcement are clear distances. Spacing of reinforcement is from center to center of reinforcement. All steel reinforcement is to be epoxy coated in accordance with Section 811.10 of the Specifications. Consider bars marked 'C' to be a stirrup for purposes of bend diameters. Non-epoxy reinforcement may be used for fabrication purposes, only, provided that the steel is not used in the top 5½ of the beam and the location of the steel is indicated on the shop drawings.

FABRICATION: Beams shall not be fabricated more than 120 days before the deck is to be poured.

GROUT: Provide non-shrink grout for anchor dowels, shear keys, and tensioning rod block-outs conforming with Section 601.03.03 of the Specifications. When side by side superstructure is utilized, grouting will be completed after lateral tension rods have been fully tightened and before leveling devices have been removed. Include the cost of furnishing and placing grout in the price of beam.

RAILING SYSTEM TYPE II: Furnish this material per these specifications.

ITEM DESCRIPTION MATERIAL SPECIFICATION C	COATING SPECIFICATION
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	A123 A123 A123 A123 A153 A153 A153 A633, Type II, Class 25 B633, Type II, Class 25 B633, Type II, Class 25 B633, Type II, Class 25 B633, Type II, Class 25

Use the current edition of the references listed below with these standards.

#### STANDARD DRAWINGS

#### SPECIAL NOTES

for Corrosion Inhibitors

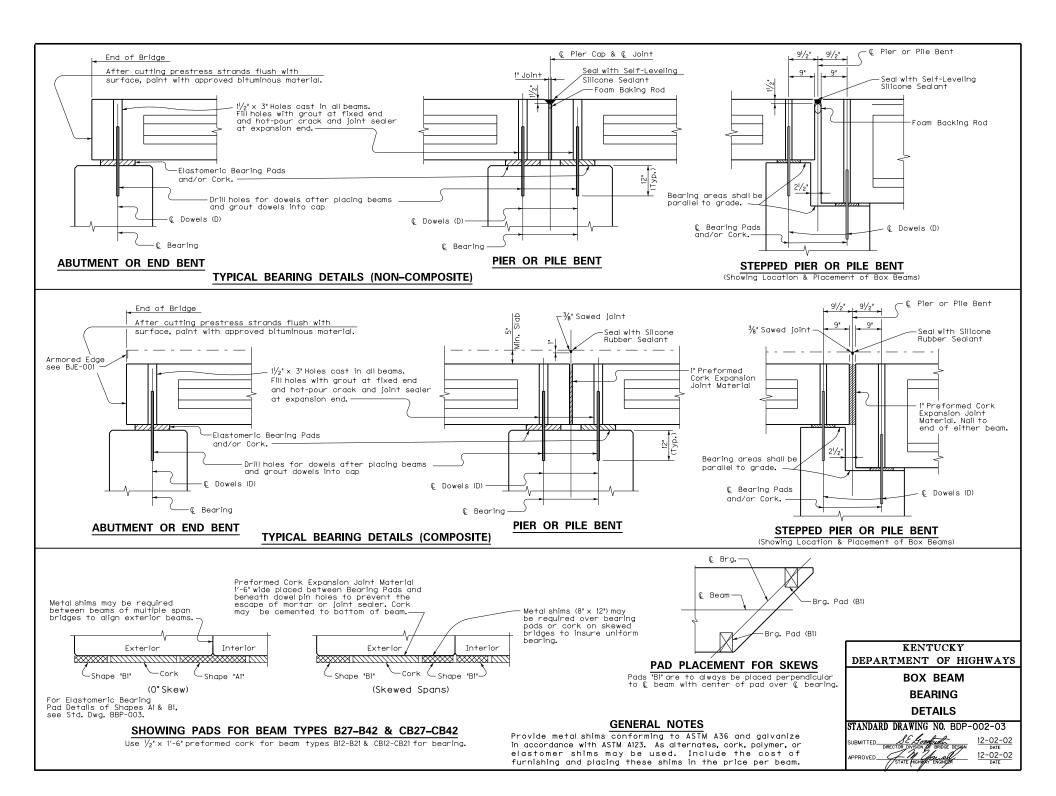
KENTUCKY DEPARTMENT OF HIGHWAYS

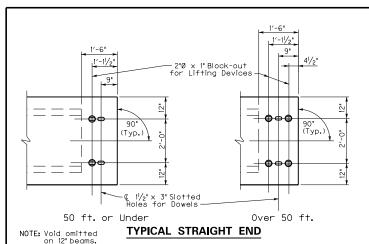
BOX BEAM
GENERAL NOTES
& REFERENCES

STANDARD DRAWING NO. BDP-001-05

UBMITTED DIRECTOR DIVISION OF STRUCTURAL DESIGN
PPROVED STATE HIGHWAY ENGINEER

12-01-15 DATE





# drain

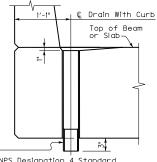
#### **VOID DRAIN DETAIL**

Locate two drains at each end of each void. Provide 1"Ø drains of a type approved by the Division of Materials.

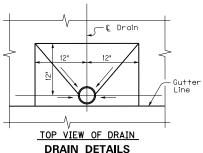
Diaphragms may be omitted if void is cut to allow drain to be encased with a minimum 2" of concrete.

#### SECTION THRU BEAM

..<del>. ()</del>



Provide drains on both sides of bridge with normal crown and on low side only for superelevated bridges. Space drains at maximum 12′-6″ on centers with a minimum of one placed each gutter line per span. Omit drains when span crosses over a highway or railroad. Include the cost of pipe and fittings in the price of beam.



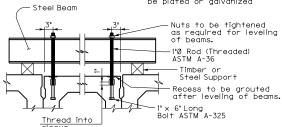
(For Spans With Curbs)

NPS Designation 4 Standard coupling and nipple in accordance with ASTM AS3. Nipple to be installed in field

#### SECTION THRU DRAIN **EXTERIOR BOX BEAM**

(Showing coupling in barrier)

Note: Inserts are to be plated or galvanized



# of exterior beam. 3/8"

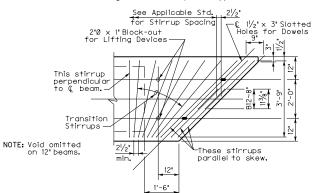
NOTE: Omit shear key on exterior face

#### SHEAR KEY DETAIL

#### See Applicable Std. for Stirrup Spacing © 1½" x 3" Slotted Holes for Dowels 2"Ø x 1" Block-out for Lifting Devices This stirrup perpendicular to C beam. -Transition Stirrups 21/2" These stirrups parallel to skew. 41/2" 1'-6"

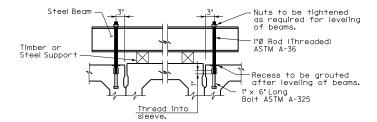
#### TYPICAL SKEWED END FOR BEAMS OVER 50 FEET

(Right Skew Shown, Left Opposite Hand)



#### TYPICAL SKEWED END FOR BEAMS 50 FEET OR LESS

(Right Skew Shown, Left Opposite Hand)



#### LEVELING DEVICE DETAILS

Locate inserts at the center of beams up to 50 ft. and at diaphragm locations of beams over 50 ft. Include the cost of materials and labor involved in leveling beams in the price for beams. Submit alternate leveling devices to the Division of Bridge Design for approval.

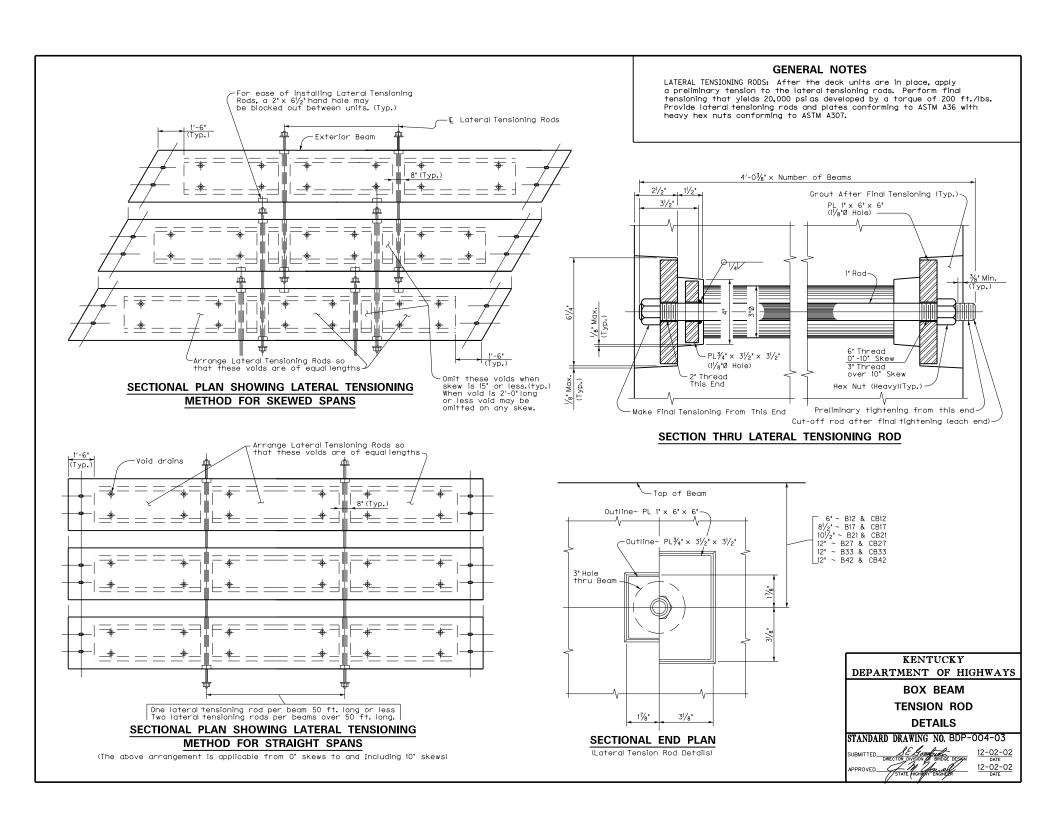
#### KENTUCKY DEPARTMENT OF HIGHWAYS

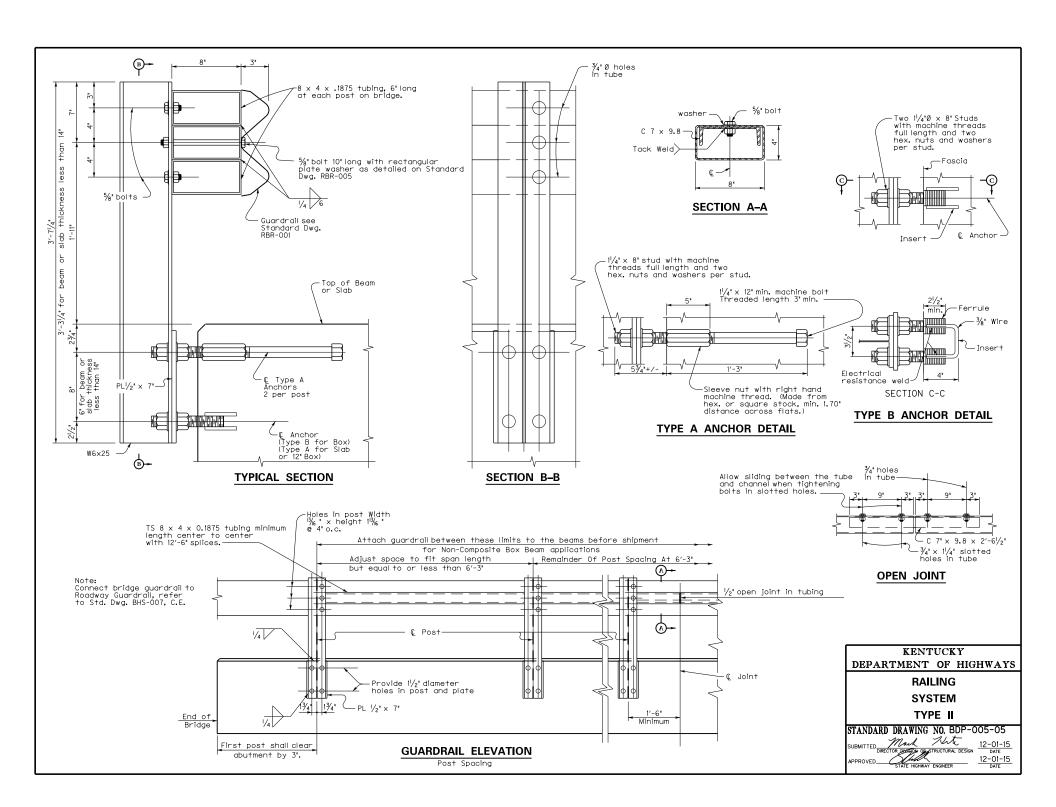
**BOX BEAM** MISCELLANEOUS **DETAILS** 

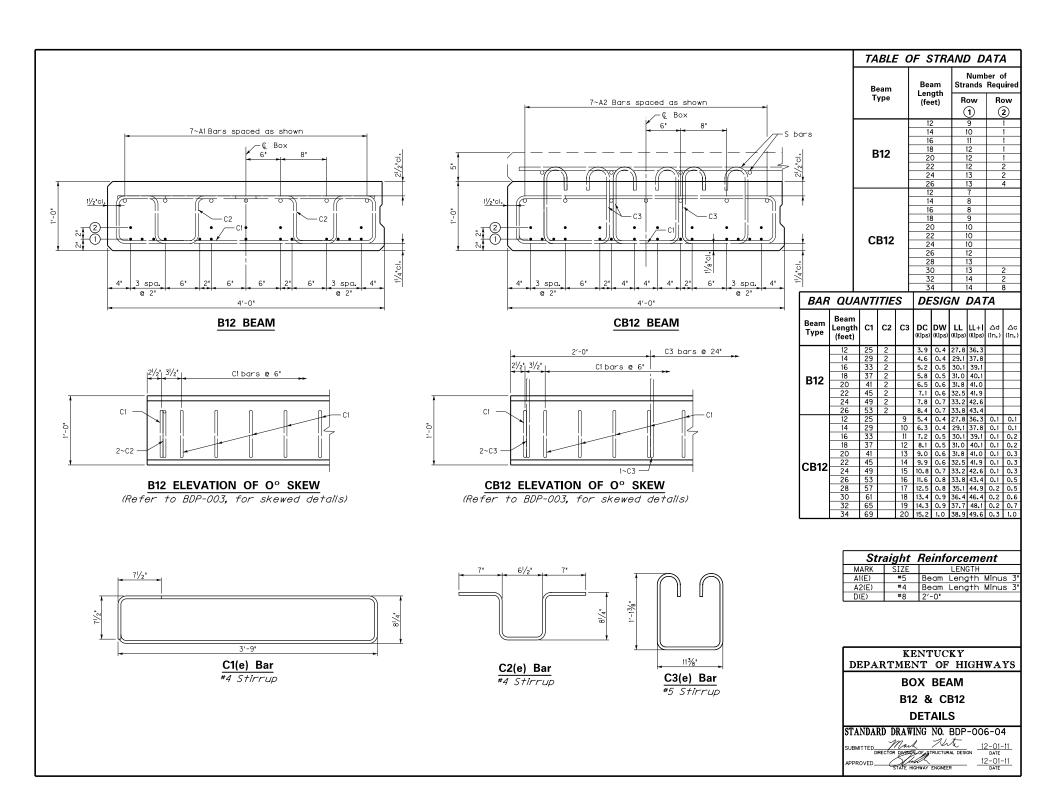
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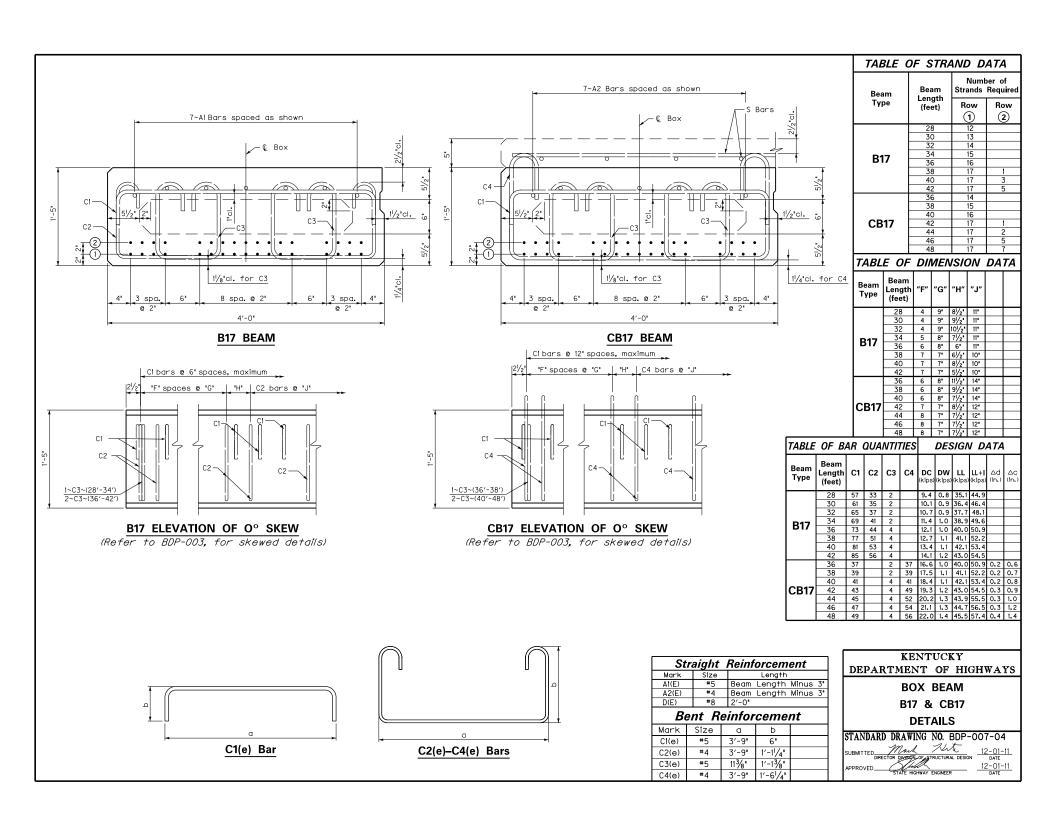
STANDARD DRAWING NO. BDP-003-03

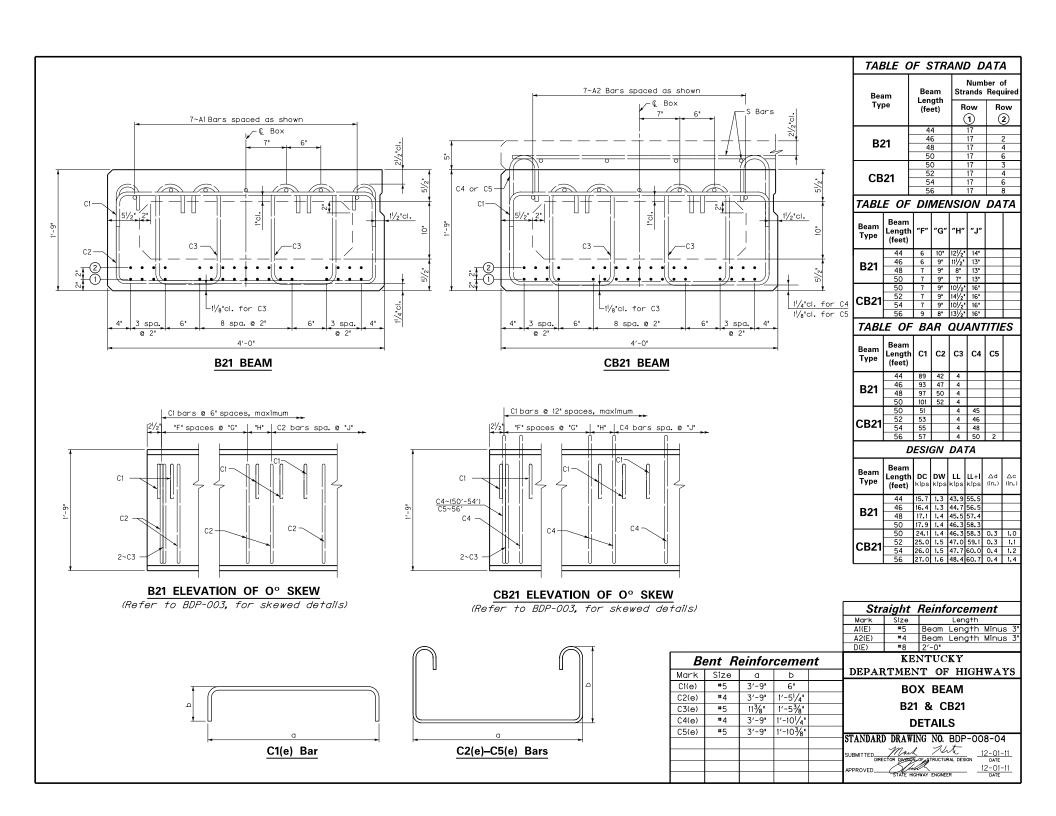
SUBMITTED Allo W Frank
DIRECTOR DIVISION OF BRIDGE DESIGN
APPROVED Machine Mathematical State (MIGHWA/ENGINEER 11-21-07 DATE

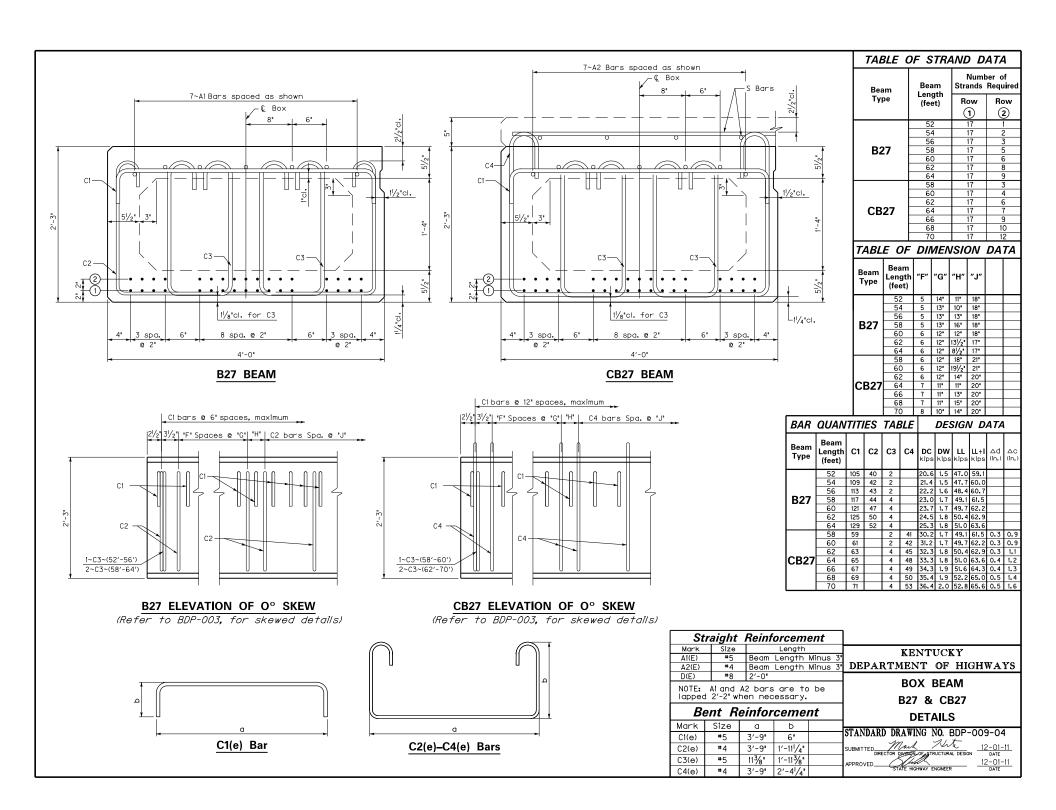


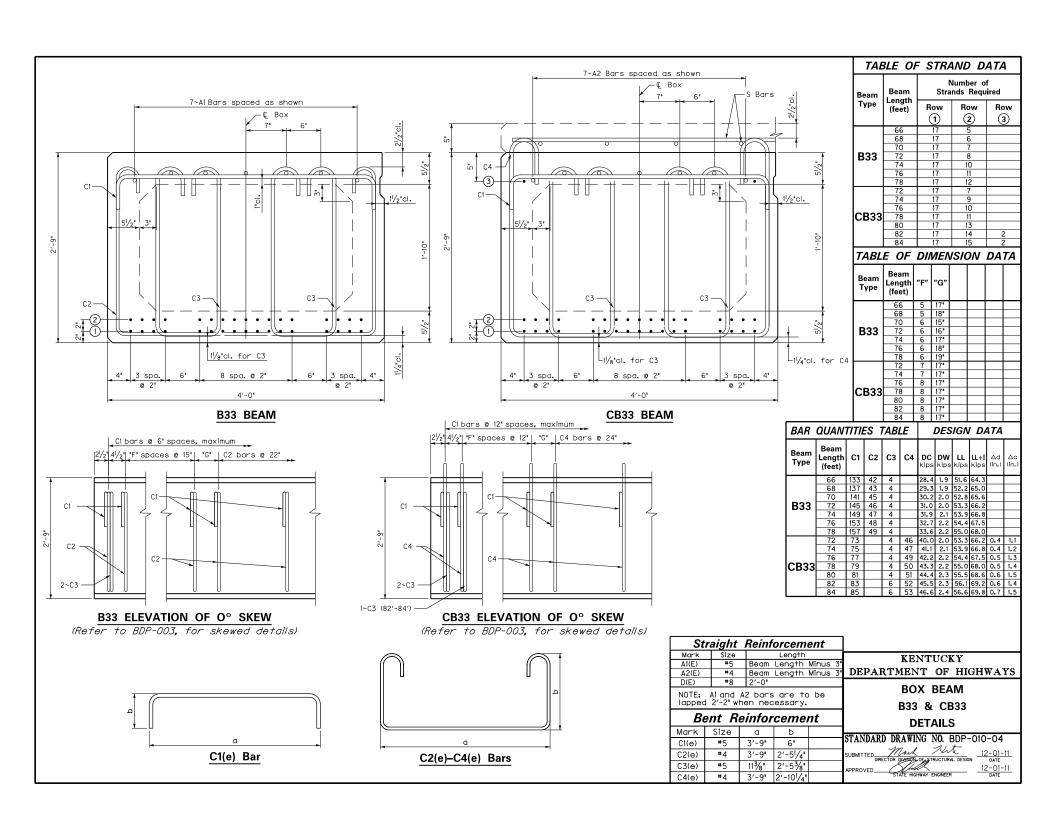


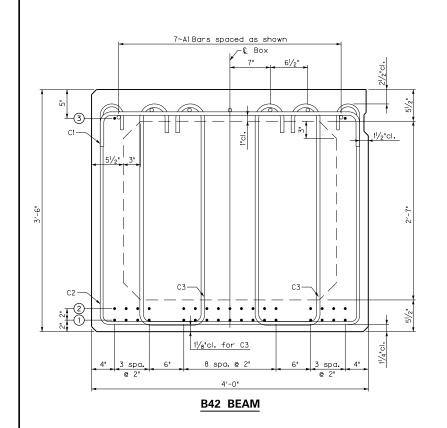


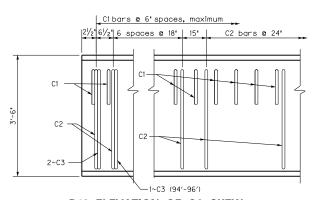












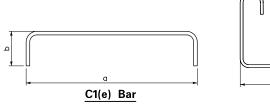
**B42 ELEVATION OF O° SKEW** 

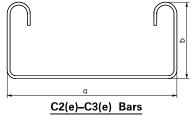
(Refer	- +0	D BDP-	<i>-003,</i>	for	skewed	details)

TABLE OF STRAND DATA							
Number of							
Beam	Beam Length		Str	ands	Req	uired	
Type	(feet)	B	low	В	low	R	ow
	(1001)	1 (	1	1 (	2	1 (	3)
	80	+	17	+	6	+	<u> </u>
	82	+	17		7	+	
İ	84	1	17	+	8	+	
İ	86	1	17		10		
B42	88		17		11		
	90	1	17		12		
İ	92		17		13		
	94		17		14		2
	96		17		15		2
BA	R QU	ΑN	TITI	ES	TA	BLE	
Beam Type	Beam Length (feet)	C1	C2	СЗ			
	80	161	46	4			
	82	165	47	4	Т		
	84	169	48	4			
	86	173	49	4	_	_	
B42	88	177	50	4	1		
	90	181	51	4	<del>                                     </del>	-	₩
	92	185	52	4	$\vdash$	$\vdash$	┢
	94 96	189 193	53 54	6	+	-	$\vdash$
		SIG			ΓΛ		_
	DE.	3/6	/V /	JAI	A		_
Beam Type	Beam Length (feet)	DC kips	<b>DW</b> kips	<b>LL</b> kips	LL+I kips		
	80	38.5	2.3	55.5			
	82	39.5	2.3		69.2		
	84	40.4	2.4		69.8		_
D 46	86	41.4	2.4	57.1			_
B42	88	42.4	2.5		70.9		
	90	43.3	2.6	58.1	71.4		-
	92	44.3	2.6	58.6		<u> </u>	<u> </u>
	94 96	45.3	2.7	59.1 59.6	72.5		-
	סכ	46.2	2.7	122.0	73.0		

Str	aight i	Reinforcement
Mark	Size	Length
A1(E)	#5	Beam Length Minus 3"
D(E)	#8	2'-0"

NOTE: Albars are to be lapped 2'-2" when necessary.





E E WITOIT HOOGGOOD JE						
KENTUCKY	Bent Reinforcement					
DEPARTMENT OF HIGHWAYS		Ь	а	Size	Mark	
BOX BEAM		6"	3′-9"	#5	C1(e)	
		3'-21/4"	3′-9"	#4	C2(e)	
B42		3'-23/8"	113%"	#5	C3(e)	
DETAILS						
STANDARD DRAWING NO BDP-011-04						
SUBMITTED MALL JULE  DIRECTOR DIMESTON OF STRUCTURAL DESIGN  DATE  12-01-11  DATE						
APPROVED STATE HIGHWAY ENGINEER DATE						

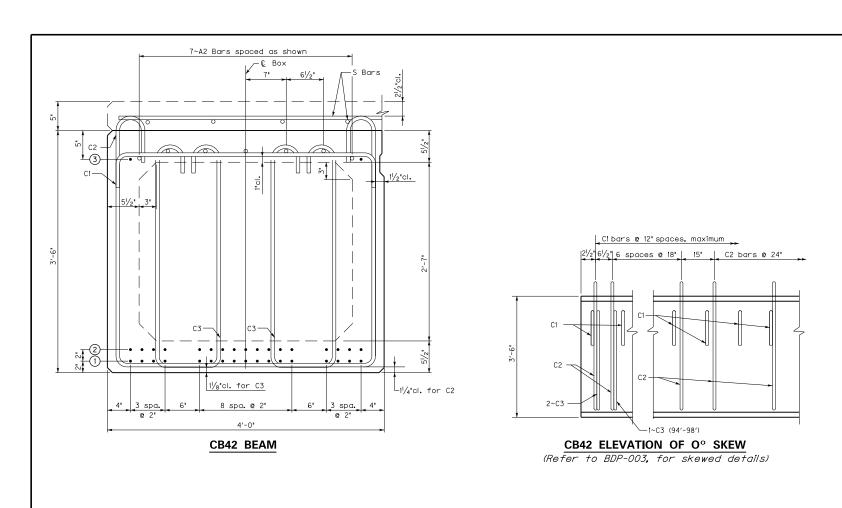


TABLE OF STRAND DATA									
Beam	Beam		Number of Strands Required						
Туре	Length (feet)	R	ow 1	FI (	low 2	1	ow 3)		
	86	+	17	+	9	+-`			
	88	+	17	+	10	+			
ŀ	90	1	17	+	12	+			
CB42	92	+	17		13	+			
0072	94	T	17		14		2		
	96	1	17		15		2		
ı	98	1	17		16		2		
BA	R QU	ΆN	TITI	ES	TA	BLE	7		
Beam Type	Beam Length (feet)	C1	C2	СЗ					
	86	87	49	4					
	88	89	50	4					
	90	91	51	4					
<b>CB42</b>	92	93	52	4					
	94	95	53	6					
	96	97	54	6					
	98	99	55	6					
	DE	SIG	N	DA7	Γ <b>A</b>				
Beam Type	Beam Length (feet)	<b>DC</b> kips	<b>DW</b> kips	<b>LL</b> kips	LL+I kips	∆d (in.)	∆c (în.)		
	86	52.1	2.4	57.1	70.3	0.4	1.0		
	88	53.3	2.5	57.6	70.9	0.5	1.1		
	90	54.6	2.6	58.1	71.4	0.5	1.2		
<b>CB42</b>	92	55.8	2.6	58.6		0.5	1.3		
	94	57.0	2.7	59.1	72.5	0.6	1.2		
	96	58.2	2.7	59.6	73.0	0,6	1.3		
	98	59.4	2.8	60.1	73.5	0.7	1.3		

Str	aight .	Reinforcement
Mark	Size	Length
A2(E)	#4	Beam Length Minus 3"
D(E)	#8	2'-0"

NOTE: A2 bars are to be lapped 2'-2" when necessary.

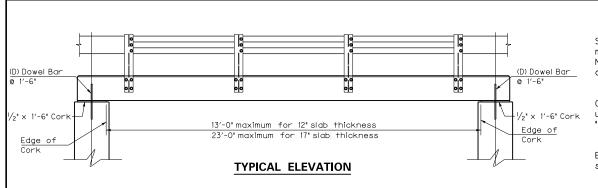
Δ	
a	
C1(e) Bar	 <u>C2</u>

		۵
_	a	
1.5	C2(e)-C3(e) Bars	<u> </u>

KEN	Bent Reinforcement						
DEPARTMENT		Ь	а	Size	Mark		
вох		6"	3′-9"	#5	C1(e)		
		3'-71/4"	3′-9"	#4	C2(e)		
(		3'-23/8"	113/8"	#5	C3(e)		
DE							
STANDARD DRAWING							
SUBMITTED Mark							
DIRECTOR DIVESION OF							
APPROVEDSTATE HIGHW							

KENTUCKY
DEPARTMENT OF HIGHWAYS
BOX BEAM
CB42
DETAILS
STANDARD DRAWING NO. BDP-012-04

SUBMITTED DIRECTOR DIMENSION STRUCTURA DESION DATE PROVIDED STATE HIGHWAY ENGINEER 22-01-11



### General Notes

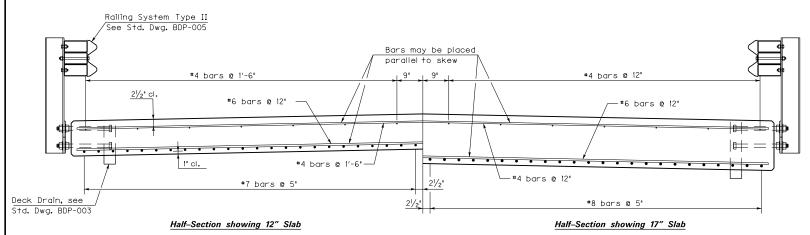
SLAB OPTION: The superstructure option shown on this Standard Drawing may be used in lieu of composite or non-composite adjacent box beams. Notify the Director of the Division of Structural Design when this option is used.

CLASS "AA" REINFORCED CONCRETE: All falsework is to remain in place until the Class "AA" Concrete compressive strength is 4000 PSI. Class "AA" Concrete is to be used throughout the superstructure.

ELEVATIONS: Determine final elevations using the elevations, slopes, and grades shown on the detailed plans.

STEEL REINFORCEMENT: Ensure steel reinforcement is ASTM A 615 Grade 60 and epoxy coated.

SURFACE FINISH: The top of the slab surface may be finished with a floated surface finish in accordance with Section 601.03.18 and textured in accordance with Section 609.03.10.

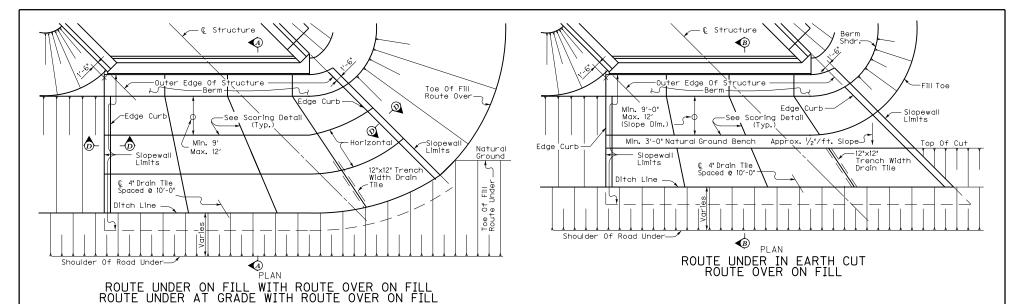


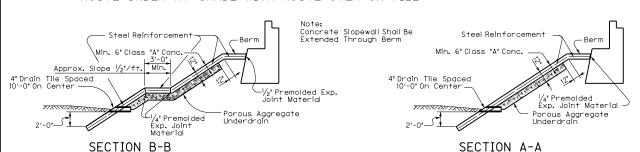
KENTUCKY
DEPARTMENT OF HIGHWAYS
SLAB BRIDGE
FOR

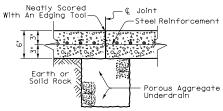
12" & 17" BEAMS

STANDARD DRAWING NO. BDP-013-03
SUBMITTED DRECTOR OF STRUCTURAL DESIGN APPROVED STATE HOWNY ENGINEER 12-01-15
12-01-15
12-01-15

TYPICAL SECTION OF CAST-IN-PLACE SLAB







#### CONSTRUCTION JOINT DETAIL

CONSTRUCTION JOINTS REQUIRED AT 21'-0' CENTERS ALONG SLOPEWALL CONSTRUCTION JOINTS PERMISSIVE AT SCORING DETAILS



#### **GENERAL NOTES**

SPECIFICATIONS: Slopewall is to be constructed according to details shown and to Section 703 of the Kentucky Department of Highways Standard Specifications for Road and Bridge Construction.

INCIDENTALS: Include the cost of steel reinforcement, drain tile, preformed expansion joint material, aggregate, excavation, and all labor and materials required to complete the work in accordance with the plans and Specifications in the price for 6' Concrete Slopewall.

ROCK EXCAVATION: Excavate the rock to plan depth and slope as near as possible to reduce the quantity of Concrete, Class 'A' required to maintain a minimum slopewall thickness. Include the cost of additional concrete required to fill voids in the rock and maintain the slopewall thickness in the bld for 6' Concrete Slopewall.

SLOPEWALL REINFORCEMENT: Use No. 4 bars at 18' centers in each direction or an equivalent area of welded deformed steel fabric to reinforce the slopewall.

 $\ensuremath{\mathsf{SKEW}}\xspace\colon \ensuremath{\mathsf{A}}$  45° Skew is detailed on this sheet. Details for other skews are similar.

Work This Drawing With Drawing No. BGX-005

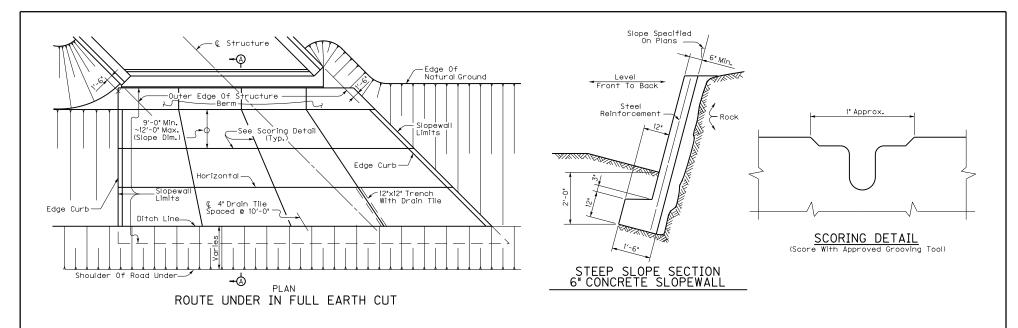
### KENTUCKY DEPARTMENT OF HIGHWAYS

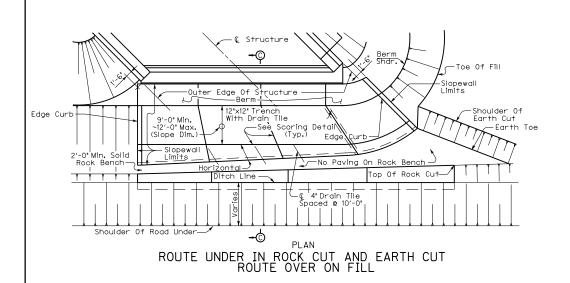
FOR GRADE
SEPARATION BRIDGES

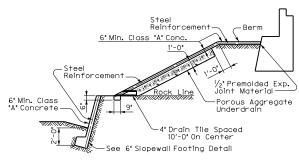
STANDARD DRAWING NO. BGX-004-09
SUBMITTED SE London 12-01-99

12-01-99

SUBMITTED SE STORE DE BRIDGE DESIGN APPROVED STATE HIGHEN ENGINEER







SECTION C-C

Work This Drawing With Drawing No. BGX-004

KENTUCKY
DEPARTMENT OF HIGHWAYS

CONCRETE SLOPEWALLS

FOR GRADE

SEPARATION BRIDGES
STANDARD DRAWING NO. BGX-005-09

SUBMITTED DIRECTOR DIVISION OF BRIDGE DESIGN
APPROVED STATE MICHAEL SHIGHER

12-01-99 12-01-99 DATE



### STENCIL FOR YEAR AND DESIGN LOADING When year only is used place year in center of plate



STENCIL FOR DRAWING NUMBER

#### GENERAL NOTES

STENCILS: Fabricate all stencils from recessed panels with beveled edges with raised letters and figures in accordance with Subsection 601.03.19 of the Specifications.

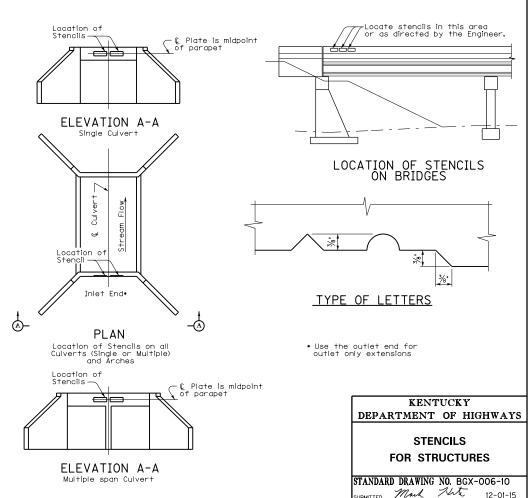
YEAR AND DESIGN LOADING STENCIL: Show the year that the contract is executed and the design load as shown on the contract plans. The design load is required on all structures classified as bridges by Subsection 101.03 of the Specifications and on other structures as referenced on plans.

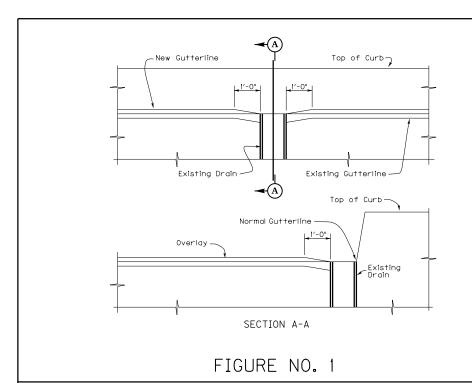
DRAWING NUMBER STENCIL: Use this stencil on all structures. The number to be placed on the stencil shall be taken from the contract plans.

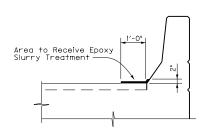
CONTRACTOR STENCIL: Place on all bridges, the name of the prime contractor and subcontractor(s), when applicable, in proximity to other stencils required.

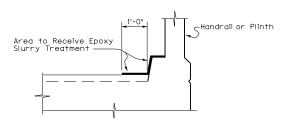


CONTRACTOR STENCIL









#### FIGURE NO. 2

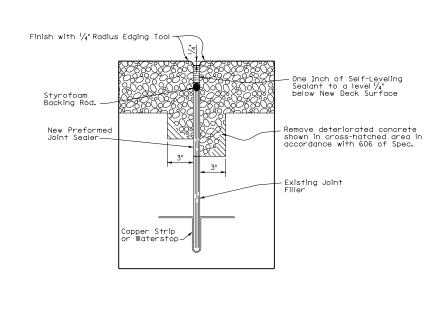
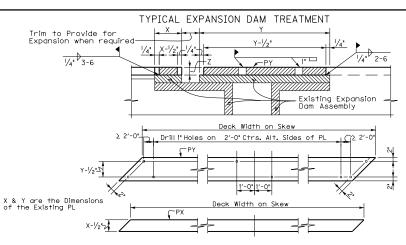


FIGURE NO. 3



Structural Steel weights given are approximate and the Contractor is responsible for all measurements.  $\,$ 

Determine dimension Z for thickness of the built-up plates as the minimum specified thickness of overlay minus 1/4.

Steel is to be furnished in 3-foot minumum lengths welded together as directed by the Engineer.

FIGURE NO. 4

#### KENTUCKY DEPARTMENT OF HIGHWAYS

BRIDGE RESTORATION
AND WATERPROOFING WITH
CONCRETE OVERLAYS

I2-01-99 DATE

Description of Soil Compactness or Consistency				
SOIL TYPE	COMPACTNESS OR CONSISTENCY	RANGE OF PENETRATION RESISTANCE	RANGE OF UNCONFINED COMPRESSIVE STRENGTH	ΑΙ
Coarse grained soils (More than half of material is larger than No. 200 sieve size.)	Very loose Loose Medium compact Compact Very compact	Less than 4 blows per ft. 4 to 10 10 to 30 30 to 50 Greater than 50	Not applicable	N S+C( O ⊕
Fine grained soils (More than half of material is smaller than No. 200 sieve size.)	Very soft Soft Medium stiff Stiff Very stiff Hard	Not applicable	Less than 0.25 tsf 0.25 to 0.50 0.50 to 1.0 1.0 to 2.0 2.0 to 4.0 Greater than 4.0	0004

				. <	Standard Penetration Test
	Unified	Soil Class	sifications	UU (psi)	Unconsolidated, Undrained T
MAJOR	DIVISION	SYMBOL	NAME	Qu (psi)	Unconfined Compressive Str
		GW	Well-graded gravels or gravel-sand mixtures, little or no fines.	w (%) RQD (%) SDI (JS)	Moisture Content Rock Quality Designation Slake Durability Index (Jar
	GRAVEL AND	GP	Poorly graded gravels or gravel-sand mixtures, little or no fines.	Rec. (%) Ø	Core Recovery Angle of Internal Friction
	GRAVELLY SOILS	GM	Silty gravels, gravel-sand-silt mixtures.	Ø c (psi)	Effective Angle of Interna Cohesion Effective Cohesion
COARSE GRAINED		GC	Clayey gravels, gravel-sand-clay mixtures.	γ Tota	Total Unit Weight Rock Disintegration Zone
SOILS		SW	Well graded sands or gravelly sands, little or no fines.	OB IB R	Overburden Bench Intermediate Bench Refusal
	SAND AND SANDY SOILS	SP	Poorly graded sands or gravelly sands, little or no fines.	NR VS (psi)	Refusal Not Encountered Field Vane Shear Strength
		SM	Silty sands, sand-silt mixtures.		
		SC SC	Clayey sands, sand-clay mixtures.		
	SILTS AND CLAYS	ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.		
FINE GRAINED SOILS	LL IS LESS THAN 50	CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays silty clays, lean clays.	Rela	tion of RQD and in situ F
	SILTS AND CLAYS	МН	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts.		90 - 100 Exce 75 - 90 Gc 50 - 75 Fc
	LL IS GREATER THAN 50	СН ////	Inorganic clays of high plasticity, fat clays.		25 - 50 Pc 0 - 25 Very
UNCLASSIFI	ED MATERIAL	NONE	Non-classified material (i.e. overburden, pavement, slag, etc.) include visual description.		

		ſ
ΑI	Activity Index	İ
LI	Liquidity Index	I
N	Penetration Resistance	ŀ
S+C(%)	Material finer than No. 200 sieve	L
0	Rockline Soundings	ſ
$\oplus$	Disturbed Sample Boring	ŀ
©	Undisturbed Sample Boring	ŀ
©	Undisturbed Sample Boring & Rock Core	ľ
0	Rock Core	
О ф	Slope Inclinometer Installation	ı
Y	typical applications: $\Diamond$ $\Diamond$ $\Diamond$ $\Diamond$	ı
$\Longrightarrow$	Approximate Footing Elevation	ı
OW	Approximate rooting Lievation	I
<i>▽</i>	7-Day (or greater) Water Table & Date	ſ
	Thin-walled Tube Sample	
<	Standard Penetration Test Sample	
	Unconsolidated, Undrained Triaxial Test	
Qu (psi)		
w (%)	Moisture Content	
RQD (%)	Rock Quality Designation	
SDI (JS)	Slake Durability Index (Jar Slake Test)	
Rec. (%)	Core Recovery	L
Ø	Angle of Internal Friction	ſ
Ø	Effective Angle of Internal Friction	
c (psi)	Cohesion	
c (psi)	Effective Cohesion	
γ	Total Unit Weight	
, RDZ	Rock Disintegration Zone	,
OB	Overburden Bench	,
IB	Intermediate Bench	
R	Refusal	Ŀ
NR	Refusal Not Encountered	ſ
		1

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		N
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LIMESTONE

SANDSTONE









TALUS OR MINE WASTE OR FILL MATERIAL



ROADWAY FILL-GRANULAR EMBANKMENT



STRUCTURE GRANULAR BACKFILL



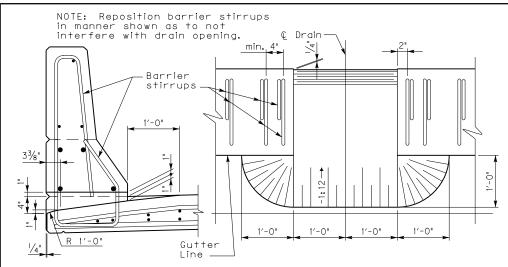
SLOPE PROTECTION

Relation of RQD	and in situ Rock Quality
RDQ (%)	Rock Quality
90 - 100 75 - 90 50 - 75 25 - 50 0 - 25	Excellent Good Fair Poor Very Poor

KENT	UCI	ΥΥ
DEPARTMENT	OF	HIGHWAYS

**GEOTECHNICAL LEGEND** 

STANDARD DRAWING NO. BGX-012-02

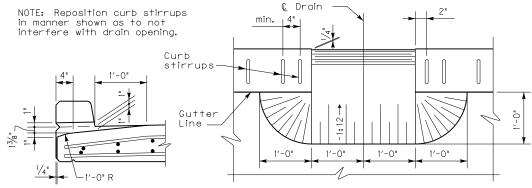


#### SECTION THROUGH DRAIN

#### PLAN OF DRAIN (Barrier not in place)

NOTE: Field bend top transverse slab reinforcement in the area of the drain to maintain  $2\frac{1}{2}$  of concrete cover through the drain. Bend reinforcement approximately 1'-0" from the gutter line. Transverse slab reinforcement adjacent to the opening is not to be bent. Longitudinal reinforcement is not to be tied to the transverse reinforcement adjacent to the drain for a distance sufficient to allow the reinforcement to sag under the bent reinforcement in the drain area.

#### THROUGH BARRIER DRAIN DETAILS

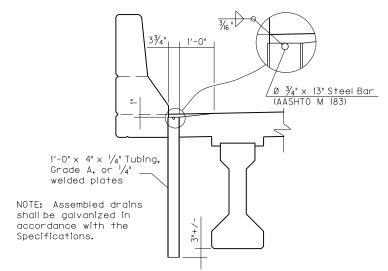


#### SECTION THROUGH DRAIN

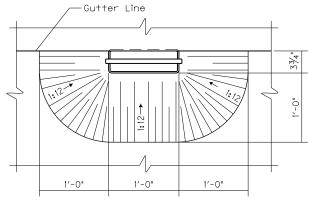
#### PLAN OF DRAIN (Curb not in place)

NOTE: Field bend top transverse slab reinforcement in the area of the drain to maintain  $2^{1}\!/_{2}^{*}$  of concrete cover through the drain. Bend reinforcement approximately 1'-0" from the gutter line. Transverse slab reinforcement adjacent to the opening is not to be bent. Longitudinal reinforcement is not to be tied to the transverse reinforcement adjacent to the drain for a distance sufficient to allow the reinforcement to sag under the bent reinforcement in the drain area.

#### THROUGH CURB DRAIN DETAILS



#### **SECTION THROUGH DRAIN**



PLAN OF DRAIN

#### THROUGH DECK DRAIN DETAILS

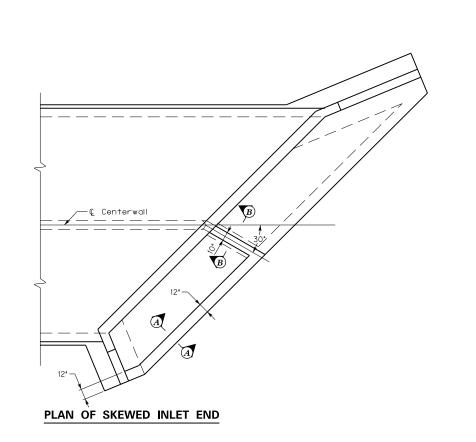
KENTUCKY DEPARTMENT OF HIGHWAYS

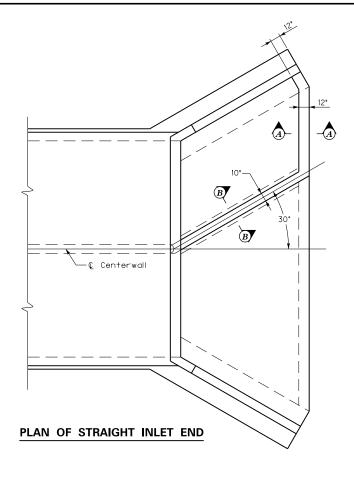
**BRIDGE DRAINS** 

STANDARD DRAWING NO. BGX-015-03

SE Goodput 12-(
OR DIVISION OF BRIDGE DESIGN

12-01-99





### Bars ⊚ 12" max. Bars @ 12" max. 2" cl. Paved Flowline -Paved Flowline Paved Flowline-1′-6" SECTION B-B SECTION A-A

#### General Notes

LOW FLOW DIVERSION CURB: Include all materials and labor required to construct the Low Flow Diversion Curb in the bid for Low Flow Diversion Curb.

DIMENSION "H": This dimension is the lesser of the wing tip elevation, one-half the barrel height opening, or as designated on the plans.

METHOD OF MEASUREMENT: The limits of the Low Flow Diversion Curb is the entire wall that is above the paved flowline, extending from the centerwall to the wing face as detailed on the Plan views, and the 6'x 1'-6' footing as shown in Section B-B.

STEEL REINFORCEMENT: All steel reinforcement is  $\pm 4$  bars in accordance with ASTM A 615, Grade 60.

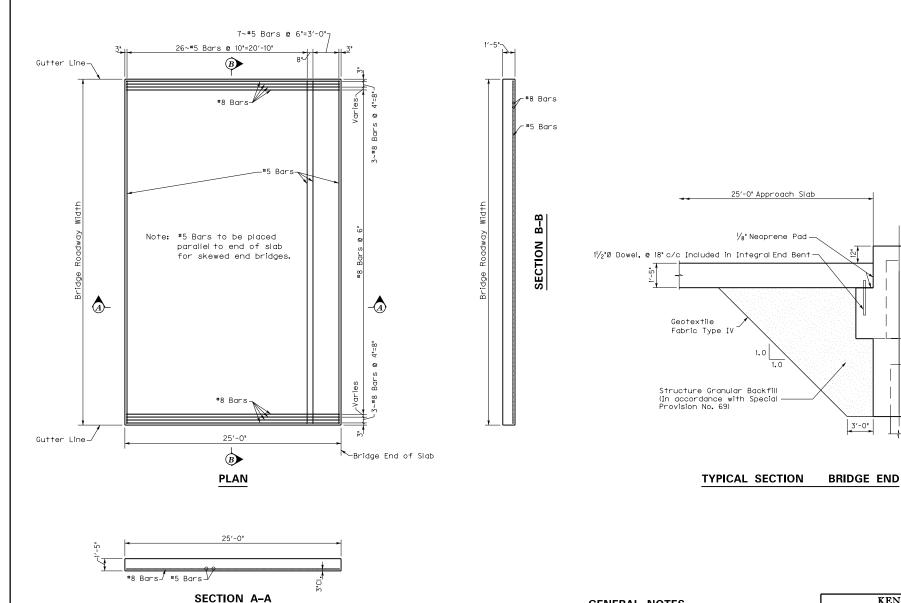
CONCRETE: Class "A" Concrete is to be used.

#### KENTUCKY DEPARTMENT OF HIGHWAYS

**LOW FLOW DIVERSION CURB** 

STANDARD DRAWING NO. BGX-016-01

12-01-99 DATE



#### **GENERAL NOTES**

CROWN: Crown shall conform to the rate of crown at the approach pavement and bridge deck. If the rate of crown at the bridge deck differs from that of approach pavement, a smooth transition shall be provided within the limits of the approach slab.

CONCRETE: Concrete shall be Class "AA".

the approach slab in the bid item for Approach Slab.

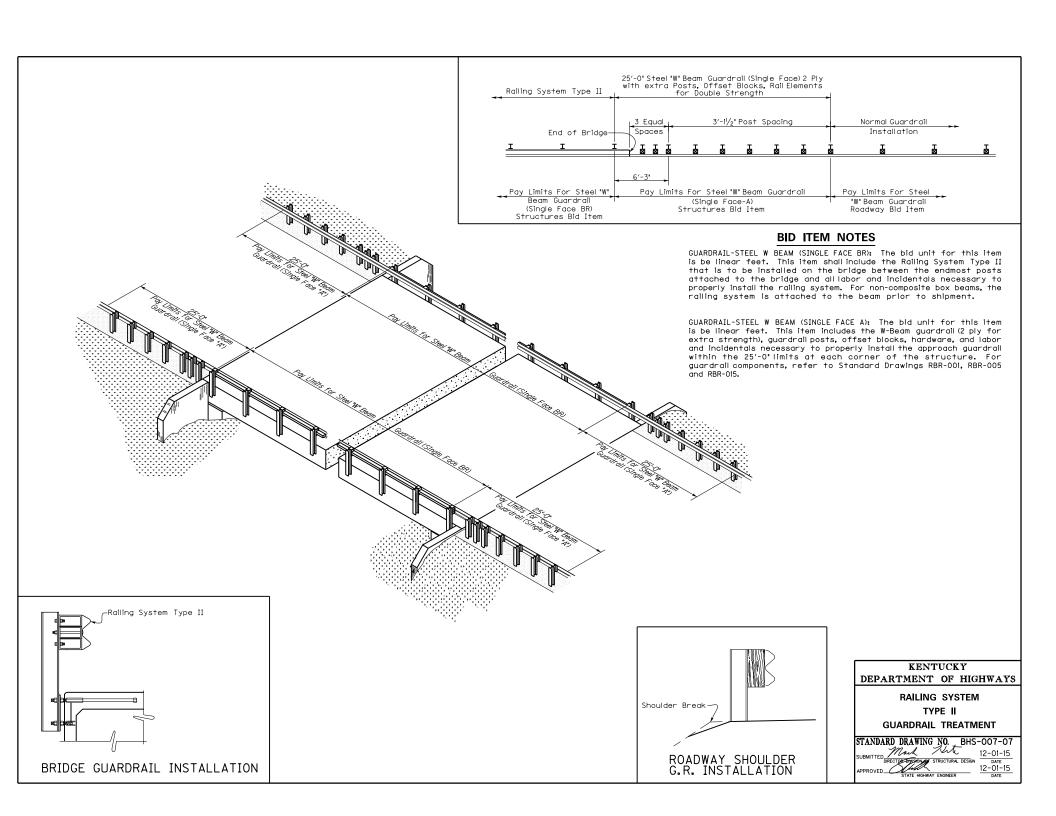
REINFORCEMENT: All steel reinforcement shall be Grade 60 and epoxy coated. | STANDARD DRAWING NO. BCX-017-02 PAYMENT: Include the cost of Class "AA" Concrete, epoxy-coated steel reinforcement, and all labor and materials required to construct

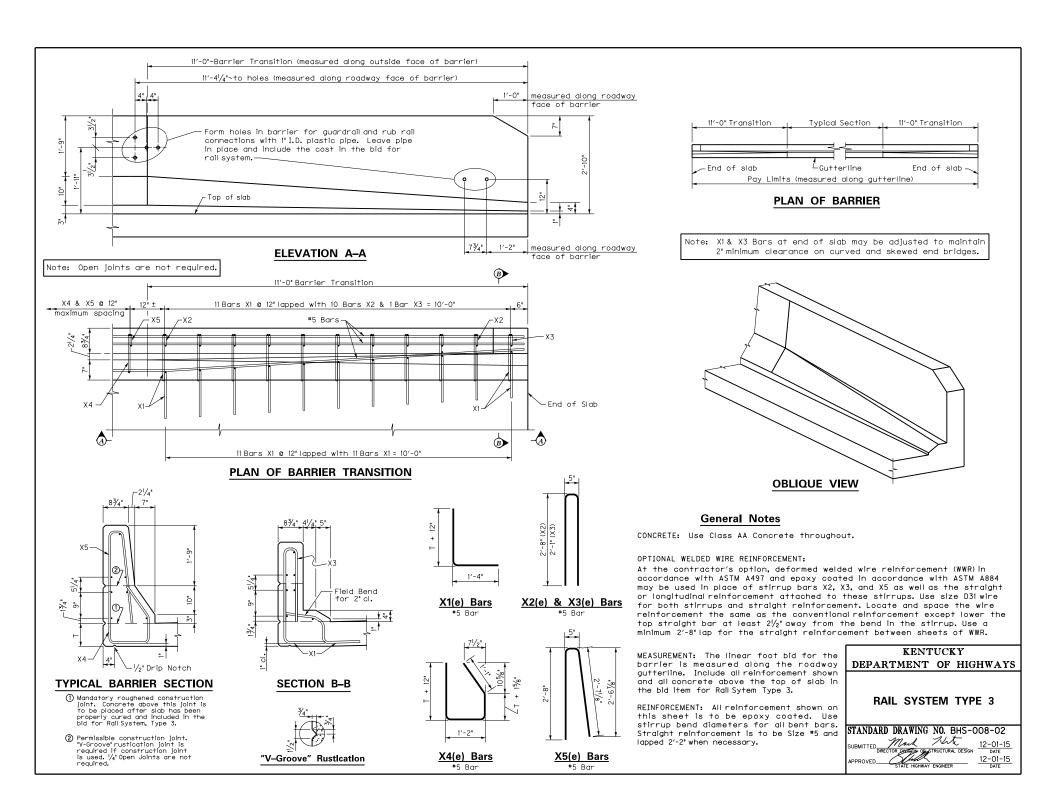
KENTUCKY DEPARTMENT OF HIGHWAYS

APPROACH SLAB

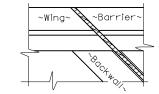
Mark Net

12-01-15 DATE





# ~Barrier~



@ Piers or Bents

<u>® Backwall Substructures</u>

#### TYPICAL BARRIER-JOINT TREATMENTS

Details are for skewed joints

# 3" maximum max

# Joint Data Dim. A Move@ 60°F ment (in) (in) 1 ½ 1 2 1 ½ 2 ½ 2

The joint seal supplied must accommodate the required movement shown. Set Dimension A with temperature change increment and as required by the manufacturer to obtain the required movement.

### 

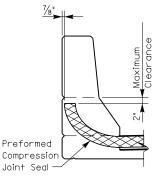
#### SECTION THROUGH JOINT

Preformed

"A" - Minimum Joint opening @ 60°F
"B" - Manufacturer's compressed
seal height 1/4".

# SECTION THROUGH ARMORED EDGE

① Assembly weight = 18.8 lbs./ft. ② Assembly weight = 12.4 lbs./ft.



SECTION THROUGH BARRIER

### **General Notes**

SPECIFICATIONS: All references to the Specifications are to the current edition of the Kentucky Department of Highways Standard Specifications for Road and Bridge Construction. All references to the AASHTO Specifications are to the current edition of the AASHTO LRFD Bridge Design Specifications.

INSTALLATION PROCEDURE: Seal the ends of the joint seal to prevent the entrance of water and foreign material.

WELDING SPECIFICATIONS: Ensure techniques and welding procedure comply with current joint specification ANSI/AASHTO/AWS DI.5 Bridge Welding Code.

MATERIAL SPECIFICATIONS: Ensure steel material is new, commercial grade steel suitable for welding. Acceptance will be based on visual inspection by the Engineer. Joint sealing material, only, is in accordance with Section 807 of the Specifications. Ensure stud shear connectors conform to ASTM A108 and A29, Grade 1015.

 ${\tt LOCATION:}\ \ \, {\tt Locate}\ \, {\tt armored}\ \, {\tt edges}\ \, {\tt and/or}\ \, {\tt expansion}\ \, {\tt dams}\ \, {\tt in}\ \, {\tt accordance}\ \, {\tt with}\ \, {\tt detail}\ \, {\tt plans.}$ 

PAINT: Clean and paint all structural steel in accordance with the Specifications, except that no field coating will be required.

SHOP DRAWINGS: Contrary to the Specifications, no shop plans are required.

BASIS OF PAYMENT: The accepted quantities of Neoprene Expansion Dam which includes the armored edges & preformed compression joint seal will be paid for at the contract unit price per linear foot for each size, measured along centerline of joint between the vertical faces of the barriers. When only an Armored Edge is required the cost of furnishing and placing the armored edge will be paid for at the contract unit price per linear foot, measured along the Armored Edge between the vertical faces of the barriers. Measure along armored edge from fascia to fascia of slab when used with Type II railing and no cure.

① Applies to 8" ② Applies to 5" slab thickness

#### **Temperature Change** Increment per 10°F Concrete Steel Span Incre-Span Incre-Length ment Length ment (f+) (in) (f+)(in) 0 - 80 0 - 60 1/32 81 - 140 61 - 100 1/16 141 - 200 101 - 140 20 I 260 141 - 180 1/8 -320

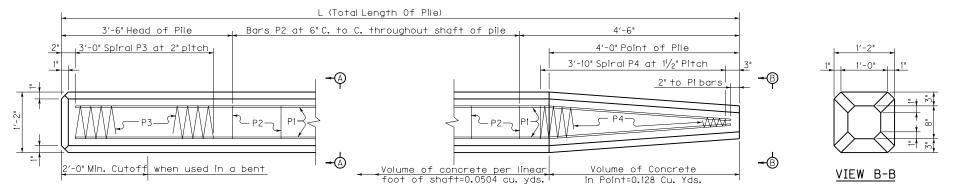
# KENTUCKY DEPARTMENT OF HIGHWAYS NEOPRENE EXPANSION DAMS AND ARMORED EDGES

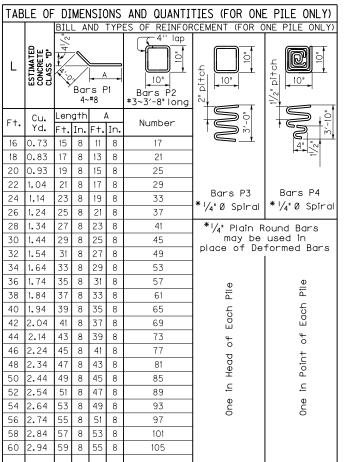
STANDARD DRAWING NO. BJE-001-13

SUBMITTED MALL STANDARD DOG FRUCTURAL DESIGN
APPROVED STAKE HIGHWAY ENGINEER

APPROVED STAKE HIGHWAY ENGINEER

APPROVED DATE

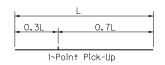


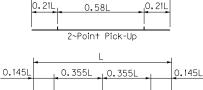


Pile lengths beyond those shown in table will have their concrete quantities adjusted to the length required.



Pick-up Method	Max. Length for 4~#8	Max. Length for 4~#9
1~Point	41 ft.	43 ft.
2~Point	58 ft.	61 ft.
3~Point	87 ft.	92 ft.







#### GENERAL NOTES

SPECIFICATIONS: Kentucky Department of Highways Standard Specifications for Road and Bridge Construction, current edition.

CONCRETE: Use class "D" concrete throughout the piles.

<u>"</u>-0-,

SECTION A-A

REINFORCEMENT: Include the cost of reinforcement in the price bid per linear foot of piles. Concrete piles must not be damaged below cut-off elevation. Concrete and spiral bars above cut-off elevation are to be removed. Bars Pl and P2 are to remain and project into structure above. Field bend these bars if necessary to maintain clearance shown on Bridge details.

PILING: Minimum penetration of all piles is 20 feet unless solid rock is encountered.

TEST PILES: Drive test piles where designated on Bridge Plans to determine the length required. Locate all test piles so they will act as a part of the piling system.

PILE CUT-OFF: No payment will be made for pile cut-off.

SPIRAL REINFORCEMENT: May be plain or deformed and have a minimum yield strength of 40,000 psi and a minimum tensile strength of 70,000 psi.

### KENTUCKY DEPARTMENT OF HIGHWAYS

14" REINFORCED CONCRETE PILE

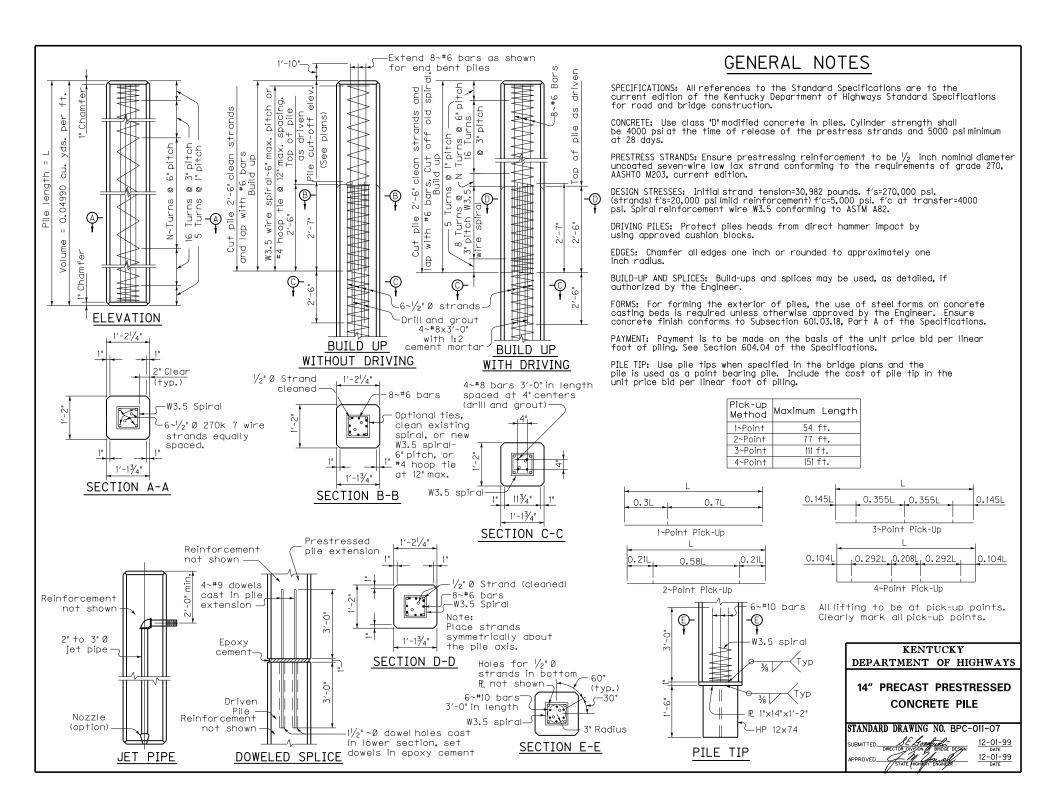
STANDARD DRAWING NO. BPC-002-08

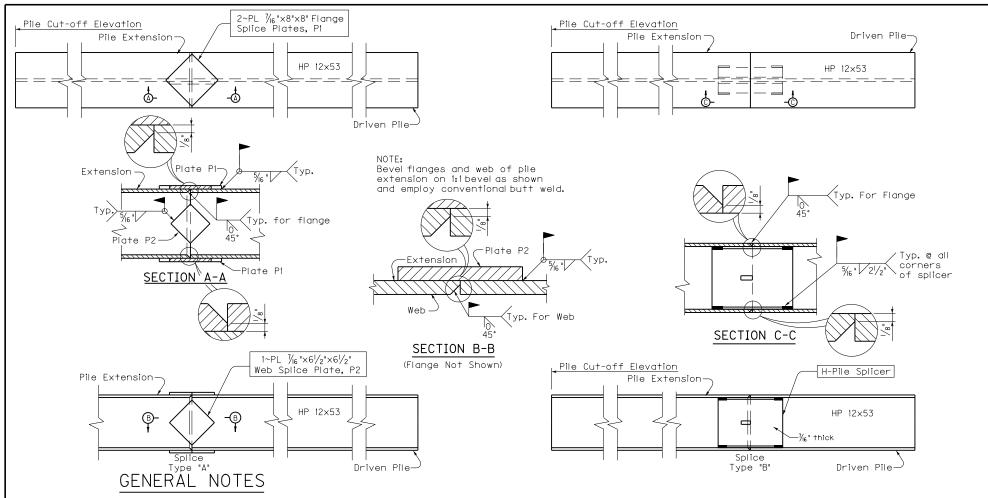
JBMITTED SE GOODALD

DIRECTOR DIVISION OF BRIDGE DESIGN

PPROVED STATE HIGHWAY ENGINER

12-01-99 DATE 12-01-99





SPECIFICATIONS: Kentucky Department of Highways Standard Specifications for Road and Bridge Construction, current edition.

MATERIALS: Ensure structural steel piles conform to A.S.T.M. A709 Grade 50, current Specifications.

SPLICE PLATES: Ensure all pile splicing options conform to A.S.T.M. A709 Grade 50, current Specifications. In lieu of Splice Option "A" or Splice option "B", splice plates may be flame cut from HP12x53 sections. If flange sections are used, the portion cut at the web must be turned outside in order to obtain a tight fit. Grind the edges smooth prior to welding.

SPLICE OPTION 'B': The pile splicer shown in the details for Splice Option 'B' may be Champion H-Pile Splicer, Model HP 30000, or an approved equal. Ensure the splicer is in accordance to the manufacturer's recommendations and subject to the Engineer's approval.

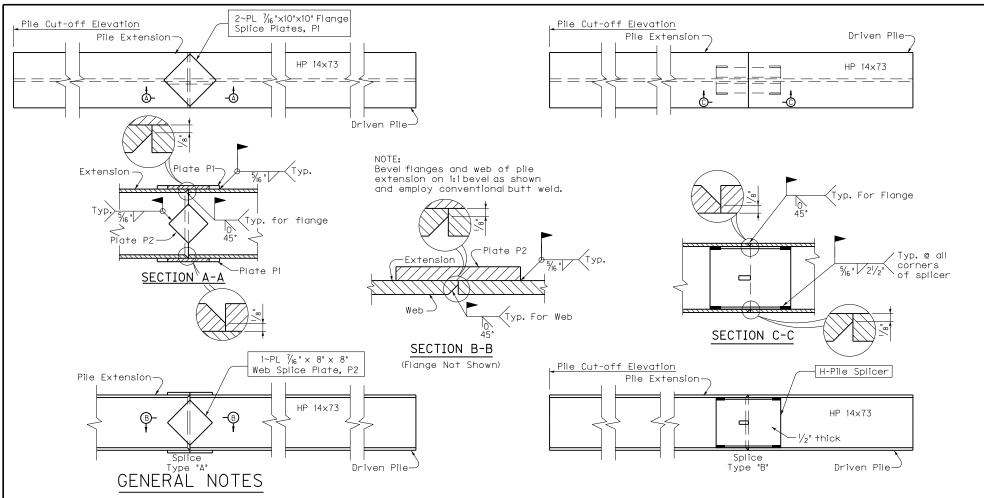
FIELD WELDS: Ensure field welding material and workmanship for all piling conforms to the current Joint Specifications ANSI/AASHTO/AWS DI.5 Bridge Welding Code. Splice piles as indicated above only when driven below cut-off elevation.

PAYMENT: Payment for the piles in accordance with plans and specifications will be made at the contract price per linear foot.

PAINT: No painting is required on steel piles.

MILL TEST REPORTS: Furnish mill test reports in triplicate to the Department showing that all materials furnished conform to the Specifications.





SPECIFICATIONS: Kentucky Department of Highways Standard Specifications for Road and Bridge Construction, current edition.

MATERIALS: Ensure structural steel piles conform to A.S.T.M. A709 Grade 50, currnet Specifications.

SPLICE PLATES: Ensure all pile splicing options conform to A.S.T.M. A709 Grade 50, current Specifications. In lieu of Splice Option "A" or Splice option "B", splice plates may be flame cut from HP14x73 sections. If flange sections are used, the portion cut at the web must be turned outside in order to obtain a tight fit. Grind the edges smooth prior to welding.

SPLICE OPTION 'B': The pile splicer shown in the details for Splice Option 'B' may be Champion H-Pile Splicer, Model HP 30000, or an approved equal. Ensure the splicer is in accordance to the manufacturer's recommendations and subject to the Engineer's approval.

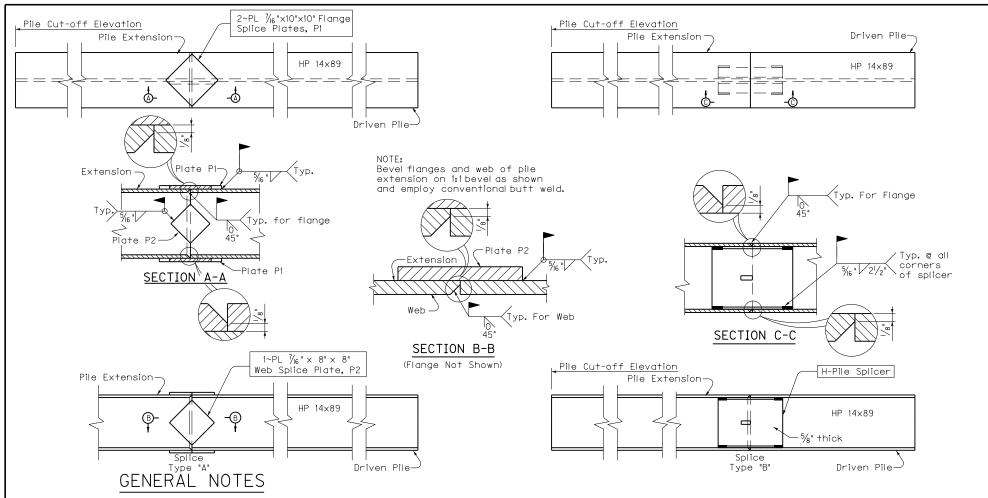
FIELD WELDS: Ensure field welding material and workmanship for all piling conforms to the current Joint Specifications ANSI/AASHTO/AWS DI.5 Bridge Welding Code. Splice piles as indicated above only when driven below cut-off elevation.

PAYMENT: Payment for the piles in accordance with plans and specifications will be made at the contract price per linear foot.

PAINT: No painting is required on steel piles.

MILL TEST REPORTS: Furnish mill test reports in triplicate to the Department showing that all materials furnished conform to the Specifications.

# KENTUCKY DEPARTMENT OF HIGHWAYS HP14x73 STEEL PILE STANDARD DRAWING NO. BPS-009-08 SUBMITED DIRECTOR PROMOTOR OF TRUCTURA. DESIGN APPROVED APPROVED 12-01-11 DATE 12-01-11 DATE 12-01-11 DATE 12-01-11 DATE 12-01-11



SPECIFICATIONS: Kentucky Department of Highways Standard Specifications for Road and Bridge Construction, current edition.

MATERIALS: Ensure structural steel piles conform to A.S.T.M. A709 Grade 50, current Specifications.

SPLICE PLATES: Ensure all pile splicing options conform to A.S.T.M. A709 Grade 50, current Specifications. In lieu of Splice Option "A" or Splice option "B", splice plates may be flame cut from HP14x89 sections. If flange sections are used, the portion cut at the web must be turned outside in order to obtain a tight fit. Grind the edges smooth prior to welding.

SPLICE OPTION 'B': The pile splicer shown in the details for Splice Option 'B' may be Champion H-Pile Splicer, Model HP 30000, or an approved equal. Ensure the splicer is in accordance to the manufacturer's recommendations and subject to the Engineer's approval.

FIELD WELDS: Ensure field welding material and workmanship for all piling conforms to the current Joint Specifications ANSI/AASHTO/AWS DI.5 Bridge Welding Code. Splice piles as indicated above only when driven below cut-off elevation.

PAYMENT: Payment for the piles in accordance with plans and specifications will be made at the contract price per linear foot.

PAINT: No painting is required on steel piles.

 $\mbox{\scriptsize MILL}$  TEST REPORTS: Furnish mill test reports in triplicate to the Department showing that all materials furnished conform to the Specifications.

# KENTUCKY DEPARTMENT OF HIGHWAYS HP14x89 STEEL PILE STANDARD DRAWING NO. BPS-011-04 SUBMITTED LORGING PRINCIPLE ADDRESSION APPROVED LORGING TRUCTURA DESIGN APPROVED LORGING TRUCTURA DE LORGING TRUCTURA DE LORGING TRUCTURA DE LORGING TRUCTURA DE LORGING TRUCTURA DE LORGING TRUCTURA DE LORGING TRUCTURA DE LORGING TRUCTURA DE LOR